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	Journal Pre-proof
1 2 3	Manuscript for CEBAMA benchmark modelling study, to be submitted July 2019 to journal of Applied Geochemistry, special issue.
4	Reactive transport modelling of a low-pH concrete / clay interface
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21	Abstract
22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38	Cement-based materials are key components in the barrier system and structural support of repositories for disposal of nuclear waste. As such, increased understanding of their long-term performance under repository conditions is paramount for the safety assessment. Quantification of the impact that cement-based materials could have on the surrounding barriers and the host rock is essential to assess long-term safety of the repository system. This interaction can impact the physical properties of the system near the interface and needs to be assessed by means of numerical modelling. A reactive transport modelling study of the interaction between a newly-developed low-pH concrete and a clay host rock (i.e. Callovo Oxfordian) over 100 000 years is presented here. The main goal is to build confidence in the consistency of the different modelling approaches and in the application of different reactive transport codes (iCP, ORCHESTRA, OpenGeosys-GEM, CORE ^{2D} , and MIN3P) to analyse the performance of the recently developed low-pH concrete within the CEBAMA project. A common setup of a reference case was established, including precipitation/dissolution reactions, redox and cation exchange processes, building upon preliminary cases of increasing complexity. In addition, a set of sensitivity cases was simulated to test the effect of key geochemical and transport parameters on the results, including the impact of porosity changes on the diffusion coefficient and electrochemical couplings. Different reactive transport codes were used in the spectmark. Overall, the results show not only the high level of
39 40	understanding of the governing processes but also the good agreement obtained with different codes, which is essential to demonstrate the applicability of reactive transport modelling to support

41 safety assessment. The sensitivity and preliminary cases modelled show that the results obtained are 42 much more sensitive to changes to transport parameters and couplings than to the different 43 modelling tools used in each case. In addition, the impact of including or not the slow kinetics of 44 dissolution of the claystone minerals is shown to be negligible in the studied scenarios.

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46 Keywords: Benchmark, reactive transport modelling, low-pH concrete, cement-clay interaction

47 **1 Introduction**

48 Cement-based materials are key components in the multi-barrier system of repositories for disposal 49 of nuclear waste (NEA, 2012). As such, increasing understanding of their long-term evolution is paramount for their performance assessment. Chemical interaction between cement-based materials 50 and the surrounding host rock is one of the most important topics. Indeed, alkaline materials such as 51 52 concrete, widely used in such disposal, are in chemical disequilibrium when in contact with clay 53 rocks, having a circumneutral pH. This disequilibrium triggers a series of interaction processes 54 driven by mass transport between the two systems. In the long term, the interface between them is 55 subjected to various physico-chemical processes that have an impact on the chemical composition of both systems and, as a result, on their physical (transport and mechanical) properties. These 56 57 various reactive transport processes develop over large ranges of space and time scales, which are not easily accessible through an experimental approach. Thus, they need to be assessed by means of 58 59 numerical modelling.

60 A large body of literature exists dealing with the modelling of cement-clay interactions. Reviews by 61 Gaucher and Blanc (2006), Savage et al. (2007), and Bildstein and Claret (2015) and Savage and Cloet (2018) are comprehensive studies of the level of knowledge and main uncertainties in this 62 field. One of the main open issues is related with the long-term performance of the clay host rock 63 64 when using recently developed low-pH concretes (e.g. Vehmas et al., 2019c). These materials were developed for different applications, such as limiting the heat of hydration or the pH of the pore 65 solution (e.g. García-Siñeriz et al., 2008). They are based on the use of substantial replacements of 66 67 cement by supplementary cementitious materials. The Horizon 2020 collaborative project 68 CEBAMA (CEment-BAsed MAterials, properties, evolution, barrier functions) was a research and innovation action granted by the EC in support of the implementation of first-of-the-kind geological 69 70 repositories for nuclear waste (Altmaier et al., 2017). One of the goals of CEBAMA was to improve the understanding of interaction processes between cementitious and clayey materials (i.e. 71 72 bentonite, clay rocks). Different approaches were used for modelling and interpretation of experimental data generated within the project and focusing on reactive transport processes that can 73 impact the physical properties of cementitious materials and their interface with clayey systems 74 75 (Idiart, 2019). Therefore, an integrated modelling study was undertaken to build confidence in the 76 consistency of the different modelling approaches. The work consisted of benchmarking the capabilities of various reactive transport codes to simulate physical and chemical processes 77 78 governing long-term interactions at a concrete-clay interface.

79 Many of the previous studies focused on the clayey system, disregarding the alteration of the 80 cementitious barrier (Savage et al., 2002; Gaucher et al., 2004; Watson et al., 2009; Fernández et al., 2010). In those studies, the cementitious system is typically replaced by a fixed concrete water 81 82 as a boundary condition. Other studies have focused on the prediction of the formation of an alkaline plume (e.g. Soler et al., 2011; Grandia et al., 2010; Sidborn et al., 2014), or the degradation 83 84 of cementitious systems by clay rock porewaters (Olmeda et al., 2017). More recently, the simultaneous interaction between clayey and cement-based materials has been paid more attention 85 (Trotignon et al., 2006; De Windt et al., 2008; Yang et al., 2008; Marty et al., 2009; Kosakowski 86 87 and Berner, 2013; Soler, 2013; Liu et al., 2014; Mon et al., 2017; Samper et al, 2018). Marty et al. 88 (2014, 2015) presented a systematic modelling study of the interaction between a Callovo-89 Oxfordian clay formation and a high-pH concrete. Benchmarking of reactive transport codes in that 90 geochemical system showed a good agreement of model results.

91 The overall evolution sequence of mineralogical assemblage of concrete made with ordinary
92 Portland cement (OPC; high-pH) during its chemical degradation is well-known (Miller et al., 2000;
93 Marty et al., 2014; Olmeda et al., 2017). However, interactions of low-pH concrete with clayey

94 materials, as studied in this paper, have been given much less attention. Thus, the goal of this study

95 is not only to build confidence in the modelling approaches by benchmarking reactive transport 96 codes, but mainly to increase the level of understanding of the alteration of low-pH cementitious 97 materials in contact with a clayey system. A reference case was simulated considering a common 98 framework of models and parameters, as well as a set of sensitivity cases for assessing the impact of 99 numerical codes specificities, material variabilities, and uncertainties on the extent of alteration. In 910 this paper, the results and main outcomes obtained with different reactive transport modelling tools 92 are presented and the implications of the study are discussed.

102 The paper is structured as follows. The conceptual model is described first, together with the 103 parameterization and the numerical implementation into the different reactive transport codes, 104 which are also briefly presented. Then, the results of the reference case obtained with the different 105 reactive transport codes are presented and compared. The main outcomes of the sensitivity analysis 106 are also addressed. Finally, the conclusions of the study are highlighted.

107 **2** Description of studied system

The studied system considers a concrete structure in contact with a clayey host rock at isothermal 108 109 (25°C) and fully water saturated conditions. Fickian diffusion is considered as the only solute transport mechanism in the reference case. All aqueous species have the same diffusion coefficient 110 in free solution. A different tortuosity is assumed for each material domain, maintaining 111 electroneutrality of the pore solution (i.e. all solutes have the same effective diffusion coefficient in 112 one given material). Both materials were considered as homogeneous and continuous porous media. 113 It is assumed that no excavation damage zone (EDZ) is present. A one-dimensional setup in 114 115 Cartesian coordinates is assumed, with a concrete thickness of 0.30 m, in contact with a large mass 116 of claystone with a thickness of 40 m. The geometry and boundary conditions are shown in Fig. 1. Fixed concentration (Dirichlet) at the clay host rock boundary corresponds to the initial porewater 117

118 composition of the claystone.



Fig. 1. Geometry and boundary conditions considered in the model.

The CEBAMA reference low-pH concrete is considered (Vehmas et al., 2017, 2019c), see Section 2.1 and Appendix A. The mineralogical composition of the clay host rock corresponds to the Callovo-Oxfordian claystone (COx), see Section 2.2, which is well characterised (Marty et al., 2014) and has been previously considered in a benchmark modelling study of the interface between a "high-pH" (OPC) concrete and COx (Marty et al., 2015). The effective diffusion coefficient and total porosity of the two domains are given in Table 1.

127 *Table 1.* Physical properties of the concrete and claystone domains considered in the model.

Material	Porosity	Pore diffusion coefficient $(D_p, m^2/s)$	Effective diffusion coefficient $(D_e, m^2/s)$
Low-pH concrete	0.04	$2.50 \cdot 10^{-11}$	$1.00 \cdot 10^{-12}$
COx claystone	0.18	$1.44 \cdot 10^{-10}$	$2.60 \cdot 10^{-11}$

Most of the simulation cases considered the thermodynamic database ThermoChimie version 9b0 (Giffaut et al., 2014). The main reason to select this database is that it includes consistent data for both, the cementitious and the clayey systems. Data for clay minerals and Portland cement hydrated

3

phases included in v9b0 were mainly selected from Blanc et al., (2015) and Blanc et al. (2010), respectively. Moreover, the database is under continuous development and regularly updated, so that future cement-clay interaction studies with this database can benefit from the present work. This version of the ThermoChimie database includes parameters for the extended Debye Hückel equation that is used calculate activity coefficients. ORCHESTRA and MIN3P use the Davies equation to calculate activity coefficients, which only requires the ion charges as input.

137 2.1 Low-pH concrete model

138 The concrete studied here is made by mixing water with cement, blast furnace slag, silica fume, 139 quartz filler, superplasticizer and aggregates with the proportions specified in Appendix A (Table 140 9). The composition of the hardened concrete can be calculated by means of thermodynamic modelling, as explained in Appendix A, starting from the raw materials. The initial porosity of the 141 142 hardened concrete adopted in the reactive transport modelling is 0.04, corresponding to the experimental value obtained by MIP (Table 1). The effective diffusion coefficient of concrete in the 143 reference case is $1 \cdot 10^{-12}$ m²/s according to Vehmas et al. (2019c) (Table 1). The mineralogical and 144 145 porewater composition after 10 years of hydration simulated with thermodynamic modelling has 146 been selected as an initial condition (see Appendix A). At that time, the remaining mass of 147 unhydrated clinkers is extremely small and can be neglected, while the slag has dissolved 148 completely. Only silica fume and quartz filler are considered in the simulations to dissolve 149 kinetically with the pH-dependent kinetic dissolution rate of quartz, r (mol/s), proposed by Palandri 150 and Kharaka (2004):

151
$$r = M_w \cdot m \cdot k \cdot A \cdot \left| 1 - \Omega^{\theta} \right|^{\eta}$$
 (2.1)

152
$$k = k_{25}^{nu} + \sum_{i} k_{25}^{i} \prod_{j} a_{ij}^{n_{ij}}$$
 (2.2)

where M_w (g/mol) is the molar mass of the mineral, m is the total mass of the mineral (mol), k is 153 the rate constant (mol/m²/s), A (m²/g) is the reactive surface area, Ω (-) is the mineral saturation 154 ratio, θ and η are rate parameters (-), superscript *nu* refers to reactions under neutral conditions and 155 superscript *i* to reactions under either acid or basic conditions, and a_{ij} is the activity of a species *j* in 156 reaction *i*. The activation energy term is equal to 1 at 25°C and is thus not included. Table 2 157 specifies kinetic parameters for quartz filler and silica fume. Surface areas were estimated from the 158 159 grain size distribution provided by the manufacturers. The aggregates are considered as chemically 160 inert.

- 161 The mineralogical and porewater compositions of low-pH concrete are given in Table 3 and Table 162 4, respectively, corresponding to the results of the hydration model (Appendix A). A volume 163 fraction of inert solid (aggregates and superplasticizer) is also considered in concrete with a value of 164 0.7748.
- In addition, the cementitious system is characterized by surface properties, modelled through cation exchange processes. Table 5 specifies the composition of the cation exchanger in equilibrium with the initial pore solution that simulates the uptake of K and Na (exchanged with Ca) in calcium silicate hydrates (C-S-H). More details are given in Appendix A. The selectivity coefficients are based on the Gaines-Thomas convention. Uptake of aluminium and magnesium in C-S-H is not considered in the model.
- The presence of redox sensitive species (i.e. iron, sulphur) in the cementitious system might influence the redox potential of cement porewater, leading to redox potentials in the range -750 mV to -230 mV in OPC (Berner, 2002). That is the case of sulphides in slag blends, and of iron, which can sorb on C-S-H phases, substitutes Al in AFm or AFt phases (e.g. ettringite and monosulfoaluminate, respectively), or precipitate as Ca-ferrites. For low-pH cementitious systems,

176 no redox potentials are available. With this in mind, and considering that the total dissolved iron 177 concentration does not exceed 10^{-7} M in cement paste (Berner, 2002), a redox potential (Eh) of ~ -178 27 mV was assumed here by considering a small amount of magnetite (Table 3) and the 179 thermodynamic equilibrium between magnetite and ferrihydrite couple at the given pH.

180 The secondary minerals allowed to precipitate are listed in Table 3. It is noted that the same set of 181 secondary minerals was considered both in the concrete and COx domains. This includes the 182 potential precipitation of quartz assuming the kinetic law defined for the COx domain (Table 6).

183 Two potentially relevant groups of phases for the studied system (i.e. low-pH concrete) that are not 184 included in the current version of the thermodynamic database are the group of Magnesium-Silicate-Hydrates (M-S-H phases) and C-(A)-S-H phases. During the course of this work, 185 thermodynamic data for the mentioned phases were reported in Lothenbach et al. (2019) and Roosz 186 187 et al. (2018). Formation of M-S-H phases at the interface between low-pH concrete and a claystone 188 has been recently identified as one of the relevant alteration processes (e.g. Dauzeres et al., 2016; 189 Mäder et al., 2017). The sink of Mg in the cementitious system is considered by other Mg-bearing phases, namely brucite and hydrotalcite. Due to significantly higher molar volume M-S-H phases 190 191 compared to other Mg-bearing phases, the impact on porosity could be non-negligible. Partial 192 substitution of Al for Si in C-S-H, so-called C-(A)-S-H phases, is also not accounted for in the 193 model. Instead, strätlingite, an Al-bearing cement hydrate is predicted to form during hydration 194 (Appendix A).

195 Table 2. Kinetic parameters for dissolution reactions in concrete (from Palandri and Kharaka, 2004).

Mineral	$A (m^2/g)$	M _w (g/mol)	k_{25}^{nu} (mol/m ² s)	k_{25}^{H+} (mol/m ² s)	n^{H+}	θ	η
Quartz_filler	0.265	60.08	$3.98107 \cdot 10^{-14}$	$5.12861 \cdot 10^{-17}$	-0.5	1	1
SilicaFume	26.087	64.531	$3.98107 \cdot 10^{-14}$	$5.12861 \cdot 10^{-17}$	-0.5	1	1

196 *Table 3.* Mineralogical composition of low-pH concrete after 10 years of hydration and COx, reaction types,

and secondary minerals considered. All reactions considered in thermodynamic equilibrium unless
 otherwise stated.

Mineral phases	mol/L concrete	mol/L COx
CSH 0.8	1.68022	0
Calcite	0.008644	5.038
Ettringite	0.011856	0
Ferrihydrite(am)	0.066384	0
Hydrotalcite	0.046836	0
Magnetite	0.000020	0
Stratlingite	0.047892	0
SilicaFume [†]	0.496924	-
Quartz filler [†]	1.748352	-
Quartz [*]	0	9.1548
Celestite	0	0.1242
Dolomite	0	0.4968
Pyrite	0	0.1908
Siderite	0	0.198
Illite_Imt-2 [†]	0	1.9386
Montmorillonite-BCCa [†]	0	0.495
Microcline [†]	0	0.2466
Ripidolite_Cca-2 [†]	0	0.0738
SiO ₂ (am)	0	0
Brucite	0	0
CSH1.6	0	0
CSH1.2	0	0
СЗАН6	0	0
C3FH6	0	0
C4AH13	0	0

C4FH13	0	0			
Ettringite-Fe	0	0			
Gypsum	0	0			
Hemicarboaluminate	0	0			
Hydrotalcite-CO ₃	0	0			
Fe(OH) ₂ (cr)	0	0			
Monocarboaluminate	0	0			
Monosulfate-Fe	0	0			
Monosulfoaluminate	0	0			
Portlandite	0	0			
Pyrrhotite	0	0			
Saponite-FeCa	0	0			
Syngenite	0	0			
[†] Dissolution kinetics; [*] precipitation kinetics					

199 *Table 4.* Initial porewater composition of the low-pH concrete after 10 years of hydration and COx
 200 claystone.

Variable	Low-pH concrete	COx claystone
pН	10.68	7.06
pe / Eh	-0.46 / -27.2 mV*	-2.84
Totals	Concentration (mol/kg water)	Concentration (mol/kg water)
Al	$1.448 \cdot 10^{-4}$	$8.504 \cdot 10^{-8}$
С	$1.506 \cdot 10^{-5}$	$3.826 \cdot 10^{-3}$
Ca	$5.237 \cdot 10^{-3}$	$7.601 \cdot 10^{-3}$
Cl	$1.000 \cdot 10^{-10\dagger}$	$4.120 \cdot 10^{-2}$
Fe	$5.447 \cdot 10^{-8}$	$4.351 \cdot 10^{-5}$
Κ	$3.420 \cdot 10^{-2}$	$5.110 \cdot 10^{-4}$
Mg	$3.736 \cdot 10^{-7}$	$5.187 \cdot 10^{-3}$
Na	$1.910 \cdot 10^{-2}$	$4.008 \cdot 10^{-2}$
S	$3.058 \cdot 10^{-2}$	$1.108 \cdot 10^{-2}$
Si	$2.021 \cdot 10^{-3}$	$1.800 \cdot 10^{-4}$
Sr	$1.000 \cdot 10^{-10\dagger}$	$2.429 \cdot 10^{-4}$
Tracer	$1.000 \cdot 10^{-3}$	0.000

 $\begin{array}{c} 201 \\ 202 \end{array}$

* pe in equilibrium with magnetite/ferrihydrite(am) pair in concrete;

[†]Very low value considered in concrete to prevent numerical instabilities when assuming a value of 0.

Table 5. Initial exchanger compositions for alkali uptake in low-pH concrete and in claystone, and
 thermodynamic equilibrium constants (Gaines Thomas convention). CEC = cation exchange capacity.

Concrete	Log K	mol/kg water	mol/L concrete
Ex2Ca	0.0	$4.444 \cdot 10^{-01}$	$1.778 \cdot 10^{-02}$
Ex2K2	0.37	$5.524 \cdot 10^{-01}$	$2.210 \cdot 10^{-02}$
Ex2Na2	0.37	$1.689 \cdot 10^{-01}$	$6.756 \cdot 10^{-03}$
Total (CEC)		2.331	0.093
Claystone	Log K	mol/kg water	mol/L COx
COx2Ca	0.7	$4.744 \cdot 10^{-01}$	$8.540 \cdot 10^{-02}$
COx2Mg	0.7	$3.282 \cdot 10^{-01}$	$5.907 \cdot 10^{-02}$
COxNa	0	$3.867 \cdot 10^{-01}$	$6.961 \cdot 10^{-02}$
COxK	1.2	$7.850 \cdot 10^{-02}$	$1.413 \cdot 10^{-02}$
COx2Sr	0.6	$1.188 \cdot 10^{-02}$	$2.139 \cdot 10^{-03}$
COx2Fe	0.8	$2.860 \cdot 10^{-03}$	$5.145 \cdot 10^{-04}$
Total (CEC)		2.1	0.378

205 2.2 Claystone model

For the Callovo-Oxfordian claystone (COx), the same physical parameters as in Marty et al. (2015) are used (porosity of 0.18 and effective diffusivity of $2.6 \cdot 10^{-11}$ m²/s, see Table 1).

6

The geochemical model of the COx is largely based on the work of Marty et al. (2015), in turn relying on the model by Gaucher et al. (2009). However, the model has been adapted to the ThermoChimie v9b0 database and incorporates two additional cations in the exchanger (Fe²⁺ and Sr²⁺). The mineralogical composition of the COx is given in Table 3. Most of the minerals are considered under thermodynamic equilibrium, except for five kinetically-controlled minerals (Table 3) based on equations (2-1) and (2-2). Among the kinetically-controlled minerals, only quartz is allowed to precipitate. The kinetic parameters are given in Table 6.

- The initial porewater composition (Table 4) is in equilibrium with the cation exchanger composition (Table 5) using thermodynamic equilibrium constants given in the same table (Gaucher et al.,
- 216 (Table 217 2009).
- Table 6. Kinetic parameters for dissolution reactions in the COx domain (from Marty et al., 2015) and for
 quartz precipitation.

Mineral	$\begin{array}{c} A \\ (m^2/g) \end{array}$	$\frac{k_{25}^{nu}}{(\text{mol/m}^2/\text{s})}$	k ^{H+} (mol/m ² /s)	n^{H+}	k ^{OH-} (mol/m ² /s)	n ^{0H–}	θ	η
Illite_Imt-2	30	$3.3 \cdot 10^{-17}$	$9.8 \cdot 10^{-12}$	0.52	$3.1 \cdot 10^{-12}$	0.38	1	1
Montmorillonite-BCCa	8.5	$9.3 \cdot 10^{-15}$	$5.3 \cdot 10^{-11}$	0.69	$2.9 \cdot 10^{-12}$	0.34	0.17	10.34
Ripidolite_Cca-2	0.003	$6.4 \cdot 10^{-17}$	$8.2 \cdot 10^{-09}$	0.28	$6.9 \cdot 10^{-09}$	0.34	1	1
Microcline	0.11	$1.0 \cdot 10^{-14}$	$1.7 \cdot 10^{-11}$	0.27	$1.4 \cdot 10^{-10}$	0.35	0.09	2.35
Quartz (precipitation)	0.05	$3.0 \cdot 10^{-12}$	-	-	-	-	4.58	0.54

220 2.3 Discretization

The same spatial discretization was used in all the 1D models implemented in all the codes. A finer 221 discretization was assumed in the concrete domain and also in the first 0.1 m of the COx domain to 222 223 avoid using elements of different size at the interface. The size of the elements is 0.02 m in the concrete domain ($0 \le x \le 0.30$), while the COx domain ($0.3 \le x \le 40.3$ m) is discretized with the 224 225 following sequence: 5 x 0.02 m, 35 x 0.04 m, 10 x 0.1 m, 10 x 0.5 m, 10 x 1 m, 10 x 2 m, and 1 x 2.5 m. The total number of elements is 60. This discretization was set as a compromise between 226 227 spatial resolution and computation time. A simulation time of 100,000 years was considered. According to the von Neumann criterion for diffusive solute transport, the time step size should 228 229 comply with the following relation:

$$230 \quad \Delta t < \frac{\Delta x^2}{3 \cdot D_p} \tag{2-3}$$

where Δt (s) is the time step size, Δx is the spatial discretization (m), and D_p is the pore diffusivity (m²/s). Given the higher pore diffusivity of the COx claystone compared to concrete, the time step size is restricted by the clay domain. The temporal discretization considers a constant time step size of 0.10 years. Some of the codes used in this study have an automatic time stepping scheme. In those cases, a maximum time step size of 0.10 years was considered.

236 2.4 Methodology

Several preliminary cases (P1-P3) of increasing complexity were implemented and simulated step by step in different reactive transport codes (Section 2.5) to finally define the full reference case (FRC). A description of the preliminary cases and the results obtained can be found as Supplementary Material to this paper. In addition, a set of sensitivity cases was modelled with some of the reactive transport codes to assess the impact of some of key parameters and processes (S1-S3). These cases are based on the FRC. Table 7 presents a summary of the main processes considered in each case.

In the sensitivity case S1, the concrete effective diffusion coefficient was reduced 10 times $(1 \cdot 10^{-13} \text{ m}^2/\text{s})$. This value is closer to the lowest range of experimental results obtained using HTO (Vopálka et al., 2019) and considers the fact that the diffusion coefficient decreases with time with continued

247 hydration (e.g. Vehmas et al., 2019c).

The full reference case does not consider diffusion-porosity coupling, i.e. changes in transport properties (D_e) as a result of porosity variations due to mineral volume changes. Feedback between chemical alteration and porosity and diffusivity was considered in the sensitivity case S2 using a linear relationship between porosity and diffusivity specified by equations (2-4) and (2-5):

$$252 D_e = D_p \phi (2-4)$$

$$253 \qquad D_p = \tau D_w$$

(2-5)

with ϕ the porosity (m³/m³) and D_p , the pore diffusivity (m²/s), defined as a function of a constant tortuosity factor (τ) and the diffusion coefficient in free water (D_w in m²/s). A minimum porosity value of 0.001 is set in the entire modelled domain to prevent full clogging.

Finally, the impact of electrochemical coupling for the transport of charged species was also 257 258 investigated in sensitivity case S3 and implemented in ORCHESTRA (see Section 2.5). This feature 259 is also available in MIN3P but could not be simulated due to limited resources. This case considered the effects of ion specific diffusion coefficients instead of an average value as in the 260 Fickian diffusion approach. Dissolved species are allowed to diffuse at different rates according to 261 their diffusivities in free solution, which were selected from the phreeqc.dat database (Parkhurst and 262 Appelo, 2013). Due to their different charges, this results in the development of local electric 263 potential gradients. These gradients have an impact on the diffusion rate of charged ions until net 264 265 charge fluxes are zero. As a result, the diffusion rate of a specific ion not only depends on its own diffusion coefficient, but on the concentration gradients of all accompanying dissolved ions. For 266 this reason, this approach is usually referred to as multicomponent transport and can be modelled 267 268 using the Nernst-Plank equations (e.g. Galíndez and Molinero, 2010):

269
$$J_i = -D_i \left(\frac{dc_i}{dx} + z_i c_i \frac{F}{RT} \frac{d\psi}{dx} \right)$$
(2-6)

In equation (2-6), subscript *i* corresponds to variables specific of species *i*, *J* (mol/s) is the diffusive flux, *D* (m²/s) is the diffusion coefficient, *c* (mol/L) is concentration, *z* is the ion valence, *F* is the Faraday constant (C/mol), *R* (J/K/mol) the constant of ideal gases, *T* is temperature (K), and ψ is the potential (V). During transport, the potential gradient is iteratively solved in ORCHESTRA to result in zero charge flux.

275	Table 7. Description	of simulation cas	es and processes	considered in this study.
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Description of simulation cases	6							
Preliminary case 1: diffusion of	a tracer f	rom con	crete int	o the cla	y rock			
Preliminary case 2: diffusion plus cation exchange and aqueous speciation reactions								
Preliminary case 3: idem case P2, and adding mineral reactions in equilibrium								
Full Reference Case: full chemical description of the system, including mineral kinetics								
Sensitivity case 1: effective diffusivity of concrete reduced by 1 order of magnitude								
Sensitivity case 2: porosity-diffusion coupling considered								
Sensitivity case 3: electrochemical coupling (i.e. Nernst-Planck equations)								
List of simulated processes P1 P2 P3 FRC S1 S2						S3		
fusion	×	×	×	×	×	×	×	
pecies + cation exchange		×	×	×	×	×	×	
n equilibrium			×	×	×	×	×	
Reaction kinetics				×	×	×	×	
Lower diffusion coefficient in concrete					×			
Porosity-diffusion coupling						×		
ponent diffusion							×	
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276 **2.5 Description of codes**

277 Five reactive transport codes have been benchmarked: iCP, OpenGeoSys-GEM, ORCHESTRA,

278 ParMIN3P-THCm and CORE^{2D} V5. Their main features are summarized in Table 8 and detailed 279 below.

280	Table 8.	Relevant features	s of reactive	transport	codes used	in this study.
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Code name	Spatial discretization	Time stepping scheme	Coupling approach for RT	Chemical equilibrium approach	Aqueous activity coefficient	TDB
iCP	FEM	Pres.	SNIA	LMA	eDH	ThermoChimie
OGS-GEM	FEM	Aut.	SNIA	GEM	eDH	Thermoddem (modified)
ORCHESTRA	FVM	Pres.	SNIA	LMA	Davies	ThermoChimie
CORE ^{2D} V5	FEM	Pres.	SIA	LMA	eDH	ThermoChimie
MIN3P	FVM	Aut.	GIA	LMA	Davies	ThermoChimie

FEM: finite element method; FVM: finite volume method; Pres.: prescribed; Aut.: automatic; SNIA: sequential non-iterative approach; SIA: sequential iterative approach; GIA: global implicit approach; LMA: law-of-mass action, GEM, Gibbs energy minimization method; eDH: Extended Debye-Hückel

281 iCP version 1.5 (interface COMSOL-PHREEQC) couples two standalone codes, the general-282 purpose software COMSOL Multiphysics® version 5.3 and the geochemical simulator PHREEQC version 3 (Parkhurst and Appelo, 2013). The interface provides a numerical platform to simulate a 283 wide range of multiphysics problems coupled with geochemistry (Nardi et al., 2014). iCP is written 284 in Java and uses the IPhreeqc C⁺⁺ dynamic library and the COMSOL Java-API. The coupling 285 approach is based on the operator-splitting (OS) technique. Conservative solute transport equations 286 coupled with other physical processes are solved by COMSOL, while equilibrium and kinetic 287 288 chemical reactions are solved by PHREEQC. iCP has been extensively used in geosciences and for 289 long-term performance of engineered barriers for nuclear waste (e.g. Idiart et al., 2019).

OpenGeoSys-GEM (OGS-GEM) is an open-source code for simulation of thermo-hydromechanical-chemical processes in porous media that couples the OpenGeoSys V5 framework with the GEMS3K thermodynamic solver (Kosakowski and Watanabe, 2014). It is based on an objectoriented Finite-Element-Method concept (Kolditz et al., 2012). Fluid flow and mass transport equations are solved by OpenGeoSys. The geochemical thermodynamic solver GEMS3K is used

for calculation of local/partial equilibria in complex heterogeneous multicomponent-multiphase systems (Kulik et al., 2013). OpenGeoSys-GEM has been applied extensively to investigate very complex geochemical reactions to assess the long-term geochemical evolution of materials and interfaces in deep geological repositories (Cloet et al., 2018; Kosakowski and Berner, 2013; Kosakowski and Watanabe, 2014; Poonoosamy et al., 2018).

300 ORCHESTRA (Objects Representing CHEmical equilibrium and TRAnsport) is written in Java (Meeussen, 2003). In ORCHESTRA, all model equations are provided as run-time input, which 301 302 means that equations can be defined directly in input files in text format. It is also possible to use 303 predefined definitions (objects) from a standard object file (e.g. reactions, minerals, diffusion, etc.). 304 ORCHESTRA has a GUI for defining chemical equilibrium systems, able to read PHREEQC 305 format thermodynamic databases. This makes ORCHESTRA a flexible tool for combining standard chemical / transport models with user defined/modified parts. This approach was followed in the 306 transport modules, where standard objects (e.g. for diffusion) where combined with user defined 307 308 expressions for feedback between porosity and tortuosity and even for implementing the effect of electric potentials on diffusion (Nernst-Planck). Kinetic reactions were calculated simultaneously 309 310 with the transport equation, using the same time-step. ORCHESTRA has been used to study solute 311 transport in cement (Sarkar et al., 2010) or cement/clay interaction (Marty et al., 2015).

ParMIN3P-THCm is the parallelized version of MIN3P-THCm, a general-purpose multicomponent reactive transport code to simulate coupled hydrogeological, thermal, and biogeochemical processes in subsurface domains with variable water-saturated conditions (Mayer et al., 2002; Mayer and MacQuarrie, 2010). A hybrid MPI and OpenMP programming approach is implemented in ParMIN3P-THCm with a domain decomposition method based on PETSc libraries (Su et al., 2017).

CORE^{2D} V5 is a code for saturated and unsaturated water flow, heat transport and multicomponent 317 318 reactive solute transport under both local chemical equilibrium and kinetic conditions in heterogeneous and anisotropic media (Samper et al., 2009; Samper et al., 2011). The flow and 319 320 transport equations are solved with Galerkin triangular finite elements and an Euler scheme for time discretization, while the chemical formulation is based on ion association theory. CORE^{2D} has been 321 widely used to model laboratory and in situ experiments (Zheng et al., 2011), the interactions of 322 corrosion products and bentonite (Lu et al., 2011) and the long-term geochemical evolution of 323 324 repositories in granite and clay (Samper et al., 2016; Mon et al., 2017; Samper et al., 2018).

325 **3** Results and discussion

The results of the full reference case (FRC) and 3 sensitivity cases are presented and discussed in this section. Supplementary data is provided separately, including the results of the preliminary simulation cases P1 to P3.

329 **3.1 Full reference case (FRC)**

The geochemical evolution of the system of the FRC is presented by means of spatial distribution profiles after 100,000 years of interaction. Comparison of results include porewater composition, exchanger evolution and mineralogical changes of the concrete and claystone domains. The simulations presented in this section were modelled using iCP, ORCHESTRA, MIN3P, OGS-GEM and CORE^{2D}. However, it is noted that the CORE^{2D} model did not consider the minerals under kinetically-controlled precipitation/dissolution. The overall evolution of the system is detailed below.

337 3.1.1 Porewater evolution

338 Spatial profiles of aqueous components and solution properties after 100,000 years are presented in 339 Fig. 2 and Fig. 3. Some species present important concentration gradients between concrete and clay porewaters, governed by equilibrium (or kinetic laws) with different sets of minerals and cation 340 distribution in the exchangers. This is the case for Al, C, Mg, Fe, Sr, or Si. Significant Mg and C 341 342 concentration gradients drive diffusion of these species from the claystone to the concrete, where 343 Mg precipitates as brucite and hydrotalcite (Fig. 6), while calcite formation near the interface is the 344 main sink of C (Fig. 7). In turn, Si diffuses from the concrete to the claystone, coupled to the 345 dissolution of C-S-H (Fig. 6), the formation of quartz (Fig. 7) and the decrease in pH in the degraded concrete region. Ca also diffuses out of the concrete structure throughout the simulation, 346 347 also driving the dissolution of cement hydrates (C-S-H, ettringite, strätlingite, see Fig. 6).

348 On the other hand, other species show small variations between concrete and clay due to their null 349 or limited role in solid-liquid interaction, at least after 100,000 years. This is the case for Cl, K, S (mainly as sulfate), or Na. Cl has no interaction with mineral phases or exchanger composition, thus 350 351 acting as a tracer. Sulphate diffuses out of the concrete domain, triggering ettringite dissolution in concrete and favouring celestite precipitation at both sides of the interface due to the ingress of Sr 352 from the claystone. However, after 100,000 years, the concentration gradient between concrete and 353 354 claystone has practically vanished. Aluminium concentration decreases in the centre part of the 355 concrete domain due to hydrotalcite precipitation in that region (Fig. 6). Changes in ionic strength 356 of the system are small and agree well between the different codes (Fig. 3).

The pH profile is a suitable indicator of chemical alteration. On the concrete side, pH values after 100,000 years present a maximum value of 10.5 in the innermost 13 cm in concrete, in equilibrium with C-S-H with Ca:Si of 0.8 (Fig. 3). The decrease from the initial value (~10.7) is due to alkali (K) diffusion to the clay. The pH decreases linearly to 10 at x=21 cm and then drops to ~7.3 towards the interface. On the COx side, pH changes are almost negligible, with a value of ~7.3 at the interface decreasing to its initial value towards the right boundary (~7.1). Thus, the maximum increase is predicted at the interface and is equal to ~0.2 pH units.

364 After 100,000 years, the redox conditions (pe) of COx remain practically unaffected (Eh ~ -170 mV) and controlled by the presence of the accessory minerals pyrite (FeS₂), celestite (SrSO₄) and 365 siderite (FeCO₃) and the precipitation of small amounts of magnetite (Fe₃O₄) in a reactive front of 366 367 ~1 m from the interface. Contrarily, an important redox gradient of between -170 mV and 190 mV is established over half of the concrete thickness (0.15 m) mainly due to the pH decrease close to 368 the claystone. This change in pH promotes the dissolution of the iron (III) phase ferrihydrite 369 370 (Fe(OH)₃) and the precipitation/dissolution of the Fe(II)/Fe(III) phase magnetite controlling the Fe(II)/Fe(III) ratio in solution and the redox of concrete. The maximum value of 190 mV at half of 371 372 the concrete thickness decreases again to its initial value of -30 mV along 7 cm due to changes in 373 the solid phases controlling the pH of the pore solution (i.e. C-S-H gel or hydrotalcite). Finally, weakly reducing conditions (Eh value of -30 mV) are kept constant in the unaltered zone of 374 375 concrete.



376 377 378 379

Fig. 2. Total aqueous species concentration (M) profiles at 100,000 years obtained with iCP, ORCHESTRA, MIN3P, OGS-GEM, and $CORE^{2D}$.



380 381 **Fig. 3.** pH, pe, and ionic strength profiles at 100,000 years obtained with iCP, ORCHESTRA, MIN3P, OGS-382 GEM, and $CORE^{2D}$.

383 **3.1.2 Cation exchanger composition**

The exchanger composition of concrete is shown in Fig. 4. After 100,000 years, K^+ concentration in the concrete exchanger decreases from $5.52 \cdot 10^{-1}$ to $\sim 10^{-4}$ M due to alkali release to the claystone, resulting in an increase in Na⁺ and Ca²⁺ (Fig. 4) with respect to the initial concentrations (Table 5). In the clay, the exchanger at 100,000 years shows a perturbation from the interaction with concrete even far from the interface (Fig. 5). The main cation exchange reaction in the clay is the increase in K⁺ from concrete leaching at the expense of a local decrease in Ca²⁺, Na⁺ and Fe²⁺. In turn, the

concentrations of all other cations show small changes. This is in agreement with the results of
 Marty et al. (2015) for a high-pH concrete – COx interface.





Fig. 5. Cation concentration (M) profiles in clay exchanger at 100,000 years obtained with iCP,
 ORCHESTRA, MIN3P, OGS-GEM, and CORE^{2D}.

400 3.1.3 Mineralogical changes

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401 Fig. 6 and Fig. 7 present the distribution profiles of mineral phases after 100,000 years of 402 interaction. The mineral degradation sequence is consistent with previous studies (e.g. Gaucher and 403 Blanc, 2006; Savage, 2011; Marty et al., 2015). After 100,000 years, the extent of the alteration front is about 15 cm in concrete (characterized by the complete dissolution of the C-S-H gel) and 20 404 405 cm in the claystone. In fact, there are two distinct zones in the COx domain: one strongly altered, 406 whose extension can be characterized by complete dolomite depletion (~ 20 cm), and another one, 407 less disturbed, corresponding to minor mineral alteration, whose extension coincides with that of 408 porosity changes (\sim 50cm, Fig. 7).

This includes dissolution of C-S-H, ettringite, hydrotalcite, strätlingite, and ferrihydrite in concrete with precipitation of brucite and calcite, and reprecipitation of hydrotalcite. In the claystone, the main changes are the dissolution of dolomite and siderite and precipitation of calcite and quartz.

412 Hydrotalcite and ettringite dissolve completely next to the interface and reprecipitate at the 413 degradation front (Fig. 6). Brucite and Fe-Ca saponite are predicted to form in concrete as 414 secondary phases, while quartz (as a result of C-S-H dissolution and consequent release of Si), 415 magnetite, celestite and calcite precipitate at both sides of the interface. A clear geochemical 416 disturbance is the formation of calcite in concrete and clay close to the interface. Calcite precipitates 417 due to the changes porewater composition: in concrete, due to the ingress of dissolved carbonate

- 418 from the COx; in the claystone, due to the diffusion of dissolved calcium and hydroxyl ions from
- 419 concrete. Calcite is the main phase responsible for porosity clogging in concrete. Quartz filler (Fig.
- 420 6) and silica fume (not shown), the two kinetically-controlled mineral phases in concrete, partially 421 dissolve at around x = 23 cm.

422 Degradation in the claystone is characterized by the dolomite and siderite distribution profiles (Fig. 423 7). Dolomite is depleted in the first 0.2 m near the interface, and partially dissolves in the next 0.6 424 m. In turn, siderite completely dissolves in the first 1.1 m, favouring pyrite and magnetite 425 precipitation. Considering the C-S-H gel and dolomite as reference minerals for concrete and COx degradation, respectively, the concrete degradation front extends over half of its thickness (0.15 m), 426 427 while in the clay the front is at 0.2 m after 100,000 years. Montmorillonite, illite, microcline, and 428 ripidolite concentrations (not shown) remain virtually unchanged after 100,000 years of interaction 429 with the low-pH concrete. This can be explained by their very slow dissolution kinetics at $pH \le$ 430 10.5.

- The FRC case does not consider changes in transport properties (porosity and diffusion coefficient) 431 432 due to mineral volume changes. Thus, no coupling with transport properties or water mass is taken 433 into account. The evolution of porosity in this uncoupled case can still be computed from mineral 434 volume fractions as a post-process of the simulations. These results are presented in Fig. 7. On the 435 concrete side, porosity remains unaltered in the innermost 5 cm. As the degradation front 436 approaches, an increase in porosity is predicted, with values of ~ 0.10 between x = 0.15 m and x = 437 0.22 m. However, as geochemical interaction proceeds, porosity decreases mainly due to calcite and 438 brucite precipitation. Negative values are predicted near the interface, showing porosity clogging. 439 On the clay side, porosity decreases over 0.5 m from the interface, although noticeable changes are 440 only predicted for the closest 3 to 7 cm, with values down to 0.12 at the interface.
- 441 Overall, the results of the FRC are very similar to case P3 without kinetically-controlled minerals 442 (see Supplementary Material), supporting the comparison of the FRC results of iCP, ORCHESTRA 443 and MIN3P with the P3 results of CORE^{2D}. Including kinetically-controlled mineral phases is, for 444 this particular case, not a key factor. Primary kinetically-controlled minerals in the clay 445 (montmorillonite, illite, microcline and ripidolite) remain unaffected for 100,000 years. In concrete, 446 silica fume and quartz filler are only dissolving locally and are replaced by quartz precipitation. One 447 of the conclusions is that kinetically-controlled minerals do not play an important role in this case.
- 448 The results obtained with the different reactive transport codes for mineral concentration profiles 449 are in good agreement, especially for concrete degradation. In the clay domain, dolomite dissolution 450 front agrees well between different codes, while mineral profiles show slight differences when 451 comparing iron-bearing minerals (magnetite, siderite and pyrite).
- 452 Some discrepancies between OGS-GEM and other reactive transport codes can be found, especially 453 in the cementitious system. This is due to the following differences in the implementation of this code. First, ThermoChimie database is not available in a version that can be used in Gibbs Energy 454 455 Minimization method (GEM). Thus, in OGS calculations a modified version of the Thermoddem 456 database (Blanc et al., 2012) was used. However, it was checked that calculated values of Gibbs 457 energy and entropy from both databases typically deviate by less than 10 J/mol. Second, OGS-GEM 458 calculates the liquid phase volume based on volume balance for each cell, even if porosity is not 459 coupled with the diffusion coefficient. In the other codes, a constant liquid phase volume was considered in the FRC. Finally, cation exchange reactions based on Gaines-Thomas convention are 460 461 used in other codes, while OGS uses an ideal solid-solution implementation based on Vanselov 462 convention.
- The results are consistent with previous studies addressing concrete/clay interaction (e.g. Gaucher et al., 2006; Savage et al., 2007; Marty et al., 2014). Recent experimental results obtained within CEBAMA (e.g. Cuevas et al., 2018; González-Santamaría et al., 2019; Mäder et al., 2017; Vhemas

466 et al., 2019c) also show qualitative agreement, e.g. in the decalcification of concrete, formation of467 calcite and Mg-rich phases near the interface.



469 ⁰ ^{0.1} _{x(m)} ^{0.2} ^{0.3} ⁰ ^{0.1} _{x(m)} ^{0.2} ^{0.3}
470 *Fig. 6. Mineral phase profiles (in mol/L medium) in the concrete domain at 100,000 years, obtained with iCP, ORCHESTRA, MIN3P, OGS-GEM, and CORE^{2D}.*472



473 fig. 7. Mineral phases (in mol/L medium) and porosity (-) profiles in concrete and clay domains at 100,000 475 years, obtained with iCP, ORCHESTRA, MIN3P, OGS-GEM, and CORE^{2D}.

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477 **3.2 Reduced concrete diffusion coefficient**

A sensitivity case (S1) assuming a lower diffusion coefficient of concrete was simulated with 478 MIN3P, $CORE^{2D}$ and iCP. A constant effective diffusion coefficient of $1 \cdot 10^{-13}$ m²/s was assumed 479 for the concrete barrier, which is one order of magnitude lower than for the FRC. Fig. 8 shows the 480 481 porosity, pH and mineralogical distribution profiles at 100,000 years obtained with the three codes. 482 Moreover, the results are compared with the FRC (iCP model results). The results of the three codes for S1 case show a very good agreement. As expected, the level of alteration at a given time is 483 484 much more limited in this case compared to the FRC due to the reduced concrete diffusivity. 485 Porosity clogging is not predicted to occur even after 100,000 years, and changes are only noticeable over a distance of 0.10 m from the interface. In the concrete domain, pH is lower than 486 487 10.5 only close to the interface (4 cm), where C-S-H gel dissolves. This contrasts with the 15 cm 488 degradation front of the FRC. Overall, the degradation sequence is qualitatively consistent with that 489 of the FRC but occurs over a much longer time scale.



490 491 Fig. 8. Distribution profiles of porosity (-), pH and C-S-H0.8 concentration (mol/L medium) at 100,000 492 years. Results of iCP, MIN3P and CORE^{2D} for S1 and of FRC (with iCP).

493 **3.3** Coupling mineral volume changes with transport properties

This sensitivity case (S2) considers the effect of mineral volume changes due to 494 495 precipitation/dissolution processes on the transport properties, i.e. porosity and diffusion coefficient. 496 This coupling has been previously studied for simple geochemical systems by Xie et al. (2015) and 497 Poonoosamy et al. (2018). The effective diffusion coefficient was chosen to depend linearly on 498 porosity changes, see equations (2-4) and (2-5). Other forms of couplings between porosity and 499 diffusivity were simulated as part of this study and the results can be found in Idiart and Laviña 500 (2019). A minimum porosity value of 0.001 is assumed to avoid complete clogging, corresponding to a minimum effective diffusion coefficient in the low-pH concrete of $2.5 \cdot 10^{-14}$ m²/s. Four codes 501 were used in this case: iCP, ORCHESTRA, OGS-GEM and CORE^{2D}. MIN3P also includes this 502 503 porosity coupling feature but could not be used for S2 due to a lack of resources. The differences between these models lie firstly, in the description of mineral reactivity. Kinetically-controlled 504 mineral phases were not included in the CORE^{2D} model. However, as shown for the FRC model 505 506 setup, only a very small impact is expected due to this simplification. Secondly, the OGS-GEM 507 model considers a fully coupled update of porosity, based on the actual volumetric fluid to solid ratio calculated by the chemical solver. 508

Fig. 9 presents the results of the four simulations together with the FRC results (iCP model results) for comparison purposes. Two variables are used as indicators of the level of alteration: porosity, which reflects the mineral volume changes, and pH. The mineralogical evolution sequence is similar to the FRC and is not repeated here. Calcite precipitation (not shown) near the interface is the main process responsible for the decrease of porosity to the residual value of 0.001. Porosity clogging is predicted after 50,000 years in iCP and ORCHESTRA (Fig. 9), virtually stopping

515 further geochemical interaction. Concrete degrades to a less extent compared to the FRC, even 516 before porosity clogging. Porosity decreases at a relatively slower rate in CORE^{2D}, although 517 differences are not significant. The reasons for this discrepancy are still not fully clear.

518 The temporal porosity changes calculated by OGS-GEM are distinctively different from the other 519 codes, since no porosity clogging is predicted after 100,000 years. This could be due to the inherent 520 difference in the porosity update formulation based on the fluid to solid ratio. As porosity decreases 521 due to mineral precipitation, the volume of reacting fluid also decreases. Thus, the amount of 522 minerals that can precipitate in the next time step also decreases. As a result, the clogging of 523 porosity is delayed. Changes in pH (Fig. 9) are more limited than in the FRC after 100,000 years due to porosity clogging. On the COx domain, pH remains below 7.2 even at the interface. In 524 525 concrete, dissolution of C-S-H is also more limited in this case, with the degradation front at 5 cm from the interface in iCP and ORCHESTRA. In turn, CORE^{2D} and OGS-GEM results show a much 526 smaller impact compared to the FRC results (Fig. 9), which is due to the relatively lower rate of 527 528 porosity decrease. The reasons for the lower pH in part of the concrete domain predicted by OGS-GEM are not completely understood but could be related to the description of cation exchange on 529 530 C-S-H in this code.





535 **3.4 Electrochemical coupling**

536 This sensitivity case (S3) was performed with ORCHESTRA. Multicomponent diffusion can have 537 noticeable impacts in systems containing ions with very different diffusion coefficients. For 538 instance, cementitious systems with hyperalkaline porewater contain large concentrations of OH⁻

539 ions which have a higher diffusion coefficient than other ions. The faster OH⁻ ions are likely to 540 induce a local potential gradient which in turn affects the diffusion of other ions.

541 The effect of multicomponent diffusion in the present study can be observed as the development of 542 an electric potential gradient over the cement-clay interface (Fig. 10). The resulting potential 543 difference (ca. 7 mV after 1 year) leads to a reduction of the diffusion rate of OH⁻ from concrete (and all other accompanying anions) and enhances transport of anions from the claystone towards 544 the concrete. For cations, the potential gradient has the opposite effect. To illustrate this, Fig. 10 545 546 shows the enhanced transport from the clay towards concrete of Cl⁻, a non-reactive anion in this 547 case, for which the effect of the induced potential gradient on diffusion is most clearly observed when comparing the same simulation with and without the electric potential gradient effect. 548 549 However, for reactive substances, such as protons (or pH) the effect is much smaller, as shown in Fig. 10. For ions that are in equilibrium with solid phases, concentration gradients are constrained 550 with mineral precipitation/dissolution fronts, so that differences between their aqueous 551 552 concentrations using both diffusion models are negligible.

553 The results show furthermore that the electric potential gradient decreases over time because of the 554 lowering of concentration gradients. This implies that the effect of electric potentials on diffusion will also decrease over time, which is illustrated by the small differences between the model results 555 556 with and without taking into account potential/charge effects. Although the effect of the 557 electrochemical coupling is clearly visible, the impact of the evolving electric potential gradients on the effective ion diffusion rates is probably small in comparison with other uncertainties. Finally, 558 559 results in terms of porosity distribution show that the effect of the potential gradient on mineral 560 precipitation and dissolution is negligible (Fig. 10), at least after 1,000 years.



561 562 Fig. 10. Electric potential (mV), dissolved chloride (mol/L), pH, and porosity distribution profiles at
563 0, 1, 10, 100 and 1,000 years. Results for ORCHESTRA with (mc) and without accounting for
564 potential/charge effects.

565 4 Conclusions

566 The results of the presented modelling study show that different reactive transport codes can be used to quantify the geochemical interaction between a low-pH concrete and a claystone to 567 essentially obtain the same results, despite of inherent differences between modelling approaches. 568 569 In all modelled cases, the differences observed between models do not have significant 570 consequences on the description of the governing processes and system evolution. This builds confidence in the application of reactive transport codes used within the CEBAMA project to 571 simulate the long-term behaviour of low-pH cementitious systems to be used for disposal of 572 573 radioactive waste, which is essential for the safety cases (Duro et al., 2019). The impact of key parameters, such as the diffusion coefficient of concrete, electrochemical couplings, and especially 574 575 the coupling between geochemical and transport parameters, has also been assessed. The impact of including or not the slow kinetics of dissolution of the claystone minerals is shown to be negligible 576 577 in the studied system. This is in part due to the low-pH nature of the cementitious system, but also to the relatively small concrete volume with respect to the clay rock volume considered in the 578 models. As a general conclusion, the modelling results of sensitivity and preliminary cases show 579 580 that the system is much more sensitive to changes in the model setup (transport parameters, 581 couplings) than to the different modelling tools used in each case.

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A subject for future research concerns the simultaneous modelling of cement hydration with reactive transport processes and water transport at the concrete/clay interface in a partially saturated system. This may be important for concrete mixes with low water-to-binder ratio, as is the case of the studied system. The role of solid solutions to represent cement hydrates, such as C-S-H with lower Ca:Si ratio than 0.8, C-A-S-H, or M-S-H could also be explored. A last issue is the assessment of more realistic couplings between mineral volume changes and their impact on the microstructure and transport properties.

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599 5 Appendix A – modelling the hydration of low-pH concrete

600 Characterization of the composition of hardened concrete after curing requires the hydration of the 601 mix to be modelled. The hydrated composition is then used as initial condition in reactive transport 602 models. To model hydration, the methodology developed by Lothenbach and Winnefeld (2006) and 603 Lothenbach et al. (2008) was followed. The model is based on coupling a set of kinetic reactions of 604 dissolution of the mix components with thermodynamic calculations. This Appendix describes the 605 cement hydration model as well as the main results in terms of phase assemblage of the cement 606 hydrates, total porosity, and porewater composition.

607 Cement hydration modelling was performed with PHREEQC v3 (Parkhurst and Appelo, 2013) 608 using the thermodynamic database ThermoChimie v9b0 (Giffaut et al., 2014), similar to the full 609 reference case. Note that the lowest Ca:Si ratio of the C-S-H model is 0.8. Data of the composition

- 610 of the raw materials available from CEBAMA (Vehmas et al., 2017, 2019b) and from literature
- (SKB, 2014) were used as input to the model. The composition of the CEBAMA reference concrete 611
- mix is detailed in Table 9. More details can be found in Vehmas et al. (2017, 2019a, 2019c). The 612 stoichiometry of the clinker phases (alite, belite, aluminate, and ferrite), silica fume (SF) and Blast 613
- Furnace Slag (BFS) are based on their experimentally-determined oxide composition (Table 10).
- 614
- 615 Table 9. Composition of CEBAMA reference concrete mix. Data from Vehmas et al. (2016, 2018, 2019c). 616 Density values from material manufacturers.

Component	Amount (kg/m ³ _{conc})	Density (kg/m ³)	Volume fraction (-)
CEM I 42.5	105	3100	0.0339
Silica fume	110	2300	0.0478
Blast furnace slag	65	2900	0.0224
Quartz filler	116	2650	0.0438
Aggregates			
0/1 mm	168	2600	0.0646
0/8 mm	770	2600	0.2962
8/16 mm	532	2600	0.2046
16/32 mm	396	2600	0.1523
Superplasticizer	16.8	1200	0.0140
Water	120	1000	0.1200
Total	2399		0.9996

617

618 Table 10. Oxide composition of the components of the CEBAMA reference mix used in the model.

Oxide	Mw (g/mol)	CEM I ^{<i>a</i>} (wt. %)	SF^{b} (wt. %)	BFS ^{c} (wt. %)
CaO	56.08	64	1.46	41.5
SiO ₂	60.08	21	93.1	32.8
Al_2O_3	101.96	3.5	1.44	10.6
SO_3	80.06	2.2	0.47	1.4
MgO	40.30	0.7	0.88	8.29
Fe ₂ O ₃	159.69	4.6	0.91	0.81
K ₂ O	94.20	0.62	1.73	0.62
Na ₂ O	61.98	0.07	-	0.62
CO_2	44.01	2.2	-	-
MnO	70.94	-	-	0.42
TiO ₂	79.87	-	-	2.04
SrO	103.62	-	-	0.05
V_2O_5	181.88	-	-	0.09
ZrO ₂	123.22	-	-	0.03
Total		98.89	99.99	99.27

^aCEM I 42.5 MH/SR/LA produced by CEMENTA AB (Anläggningscement) (SKB, 2014). ^bSF from Vhemas et al. (2019), ^cBFS measured at VTT (Finland).

619 Cement hydration is assumed to occur under atmospheric conditions at 25°C and 100% relative

620 humidity (i.e. unlimited supply of water for hydration). Redox reactions are not considered in the

simulation. Empirical expressions of dissolution rates of clinker phases follow from Parrot and 621

Killoh (1984). The dissolution rate of the BFS is taken from Schöler et al. (2017), while those of SF 622

623 and quartz filler correspond to that of quartz from Palandri and Kharaka (2004). The latter is based

on a pH-dependent transition-state-theory formulation. The specific surface areas of the BFS, SF, 624

and quartz filler are either specified directly by the providers or derived from their particle size 625

626 distribution. Apart from these kinetically-controlled reactions, all other chemical reactions are considered under thermodynamic equilibrium. The aggregates and superplasticizer are considered 627 as chemically inert. The alkali uptake in C-S-H phases (Na⁺ and K⁺) is considered using a cation 628 629 exchange model based on Missana et al. (2017), with a cation exchange capacity (CEC) that 630 depends on the concentration of C-S-H phases and their Ca/Si ratio (Hong and Glasser, 1999). On 631 the other hand, aluminium and magnesium uptake in C-S-H is not considered in the simulations. 632 Finally, calcium aluminosilicate hydrate (C-A-S-H) or magnesium silicate hydrate (M-S-H) phases 633 are not included in the thermodynamic database.

634 The dissolution of the clinker, binder components, and quartz filler are shown as a function of time in Fig. 11, together with the precipitation of cement hydrates, in terms of volume fraction (m³ per 635 m³ of concrete). An almost complete dissolution of the clinker phases and BFS occurs within the 636 first 100 days. On the other hand, SF rapidly dissolves at the beginning, but after 100 days it slows 637 down significantly. This is due to the drop in pH of the pore solution until reaching a constant value 638 639 in equilibrium with C-S-H phases that form with a Ca:Si ratio of 0.8. Finally, dissolution of quartz filler is very limited, due to the lower surface area compared to silica fume (see Table 2). The final 640 641 phase assemblage is composed of unhydrated SF, C-S-H with Ca:Si ratio of 0.8, strätlingite (Al-642 bearing phase), ettringite (sulphate-bearing phase), and ferrihydrite (Fe-bearing phase). The minerals determined experimentally in the hydrated system are unreacted SF and quartz, C-S-H 643 with a Ca:Si ratio between 0.5 and 0.7, C-A-S-H phases with an Al:Si ratio of 0.05, ettringite, and 644 645 small amounts of BFS and clinker phases, while absence of portlandite was experimentally 646 confirmed (Vehmas et al., 2019c).



647

Fig. 11. Dissolution of the clinker and binder phases, precipitation of main cement hydrates (volume fraction
in %) and concrete porosity (-) as a function of hydration time (in days). The black solid line corresponds to
porosity.

The evolution of porewater composition and pH is shown in Fig. 12. The pH of the pore solution fluctuates between 12.5 and 13 at hydration times < 10 days. This short period is followed by large variations in pH until reaching a value of ~10.7 at long hydration times. The evolution of the Ca:Si ratio of the C-S-H has a large impact on the pH of the pore solution as portlandite is already

dissolved after 5 days. The two abrupt changes in pH at 10-12 days and 80-100 days correspond to
changes in the Ca:Si ratio of C-S-H, from 1.6 to 1.2, and from 1.2 to 0.8. The values obtained
experimentally using the ex-situ leaching method (Alonso et al., 2012) are slightly higher beyond
100 days of hydration, between 11.3 and 11.7 (Vehmas et al., 2019c).

The alkali uptake in the C-S-H phases is also influenced by the pH of the pore solution. Mg^{2+} and Fe³⁺ concentrations in the aqueous solution remain low (< 10⁻⁶ M) and are controlled by the presence and the low solubility of hydrotalcite and ferrihydrite, respectively. In turn, sulphate concentration remains below $3 \cdot 10^{-2}$ M, controlled by ettringite solubility, while the aluminium concentration is controlled by strätlingite solubility. At pH \leq 10.7, the dissolution of SF and quartz filler is significantly lowered, and the system virtually reaches equilibrium. In fact, changes observed between hydration times of 10 years, 100 years or 1,000 years are extremely small.



666

667 *Fig.* 12. Evolution of aqueous composition and pH as a function of hydration time.

668 The total porosity, denoted as ϕ_{tot} and calculated with equation (A.1), is shown as a function of 669 hydration time in Fig. 11. The value of total porosity after long hydration times is around 0.063. 670 This value is relatively larger than the value obtained experimentally using mercury intrusion 671 porosimetry (MIP), i.e. 0.038 (Vehmas et al., 2019b, 2019c) and smaller than the values measured 672 with the kerosene method (0.125) or calculated considering the grain density (0.128) (Vehmas et al. 673 2019c).

674
$$\phi_{tot} = \frac{V_{b,0} + V_{w,0} - V_{b,t} - V_{hyd,t}}{V_{b,0} + V_{w,0} + V_{agar} + V_{sp}}$$
(A.1)

675 In equation (A.1), V stands for volume (litre) and subscripts b, w, hyd, aggr, and sp stand 676 respectively for unhydrated binder, water, cement hydrates, aggregates, and superplasticizer 677 (considered inert), respectively, while subscripts 0 and t refer to initial values and values at time t.

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923 7 Supplementary material

924 **7.1 Thermodynamic data of minerals**

925 **Table 11.** Thermodynamic data from ThermoChimie database (Giffaut et al., 2014) and molar volumes (M_v) 926 of minerals (from Thermoddem database, Blanc et al., 2012).

Mineral phases	M _v (cm ³ /mol)	Formula	Log K
CSH 0.8	59.29	$Ca_{0.8}SiO_{2.8}:1.54H_2O = 0.8Ca^{+2} - 1.6H^{+} + 1H_4(SiO_4) + 0.34H_2O$	11.05
Calcite	36.93	$CaCO_3 = 1Ca^{+2} + 1CO_3^{-2}$	-8.48
Ettringite	710.32	$Ca_{6}Al_{2}(SO_{4})_{3}(OH)_{12}:26H_{2}O = 6Ca^{+2} + 2Al^{+3} - 12H^{+} + 3SO_{4}^{-2} + 38H_{2}O$	56.97
Ferrihydrite(am)	34.36	$Fe(OH)_3 = 1Fe^{+3} - 3H^+ + 3H_2O$	2.54
Hydrotalcite	227.36	$Mg_4Al_2(OH)_{14}:3H_2O = 4Mg^{+2} + 2Al^{+3} - 14H^+ + 17H_2O$	73.74
Magnetite	44.52	$Fe_3O_4 = 2Fe^{+3} + 1Fe^{+2} - 8H^+ + 4H_2O$	10.41
Stratlingite	215.63	$Ca_2Al_2SiO_3(OH)_8:4H_2O = 2Ca^{+2} + 2Al^{+3} - 10H^+ + 1H_4(SiO_4) + 11H_2O$	49.66
SilicaFume	28.06		-
Quartz filler	22.69	$\mathrm{SiO}_2 = 1\mathrm{H}_4(\mathrm{SiO}_4) - 2\mathrm{H}_2\mathrm{O}$	-3.74
Quartz	22.69	$SiO_2 = 1H_4(SiO_4) - 2H_2O$	-3.74
Celestite	46.25	$Sr(SO_4) = 1Sr + 2 + 1 SO4 - 2$	-6.62
Dolomite	64.3	$CaMg(CO_3)_2 = 1 Ca^{+2} + 1 Mg^{+2} + 2 CO_3^{-2}$	-17.12
Pyrite	23.94	$\text{FeS}_2 = 1 \text{ Fe}^{+2} + 2 \text{ HS}^{-} - 1 \text{ H}_2\text{O} + 0.5\text{O}_2$	-58.78
Siderite	29.38	$Fe(CO_3) = 1 Fe^{+2} + 1 CO_3^{-2}$	-10.80
Illite_Imt-2	139.18	$ (Na_{0.044}K_{0.762})(Si_{3.387}Al_{0.613})(Al_{1.427}Fe_{0.292}Fe_{0.084}Mg_{0.241})O_{10}(OH)_2 = 0.241Mg^{+2} + 0.762K^{+} + 0.044Na^{+} + 0.292Fe^{+3} + 0.084Fe^{+2} + 2.040Al^{+3} - 8.452H^{+} + 3.387H_4(SiO_4) - 1.548H_2O $	11.54
Montmorillonite-BCCa	132.48	$Ca_{0.17}Mg_{0.34}Al_{1.66}Si_4O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 1.66Al^{+3} - 6H^{+} + 4H_4(SiO_4) - 4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 1.66Al^{+3} - 6H^{+} + 4H_4(SiO_4) - 4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 1.66Al^{+3} - 6H^{+} + 4H_4(SiO_4) - 4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 1.66Al^{+3} - 6H^{+} + 4H_4(SiO_4) - 4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 6H^{+} + 4H_4(SiO_4) - 4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 6H^{+} + 0.4H_4(SiO_4) - 0.4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 6H^{+} + 0.4H_4(SiO_4) - 0.4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 0.4H_4(SiO_4) - 0.4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 0.4H_4(SiO_4) - 0.4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 0.4H_4(SiO_4) - 0.4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.34Mg^{+2} + 0.66Al^{+3} - 0.4H_4(SiO_4) - 0.4H_2O_{10}(OH)_2 = 0.17Ca^{+2} + 0.4H_4(SiO_4) - 0.4H$	4.20
Microcline	108.74	$KAlSi_{3}O_{8} = 1 K^{+} + 1 Al^{+3} - 4 H^{+} + 3 H_{4}(SiO_{4}) - 4 H_{2}O$	0.05
Ripidolite_Cca-2	211.92	$ (Mg_{2.964}Fe_{1.712}Fe_{0.215}Al_{1.116}Ca_{0.011})(Si_{2.633}Al_{1.367})O_{10}(OH)_8 = 0.011Ca^{+2} + 2.964Mg^{+2} + 0.215Fe^{+3} + 1.712Fe^{+2} + 2.483Al^{+3} - 17.468H^{+} + 2.633H_4(SiO_4) + 7.468H_2O $	61.35
SiO ₂ (am)	29.00	$SiO_2 = 1 H_4(SiO_4) - 2 H_2O$	-2.71
Brucite	24.63	$Mg(OH)_2 = 1 Mg^{+2} - 2 H^+ + 2 H_2O$	17.10

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CSH1.6	84.68	$Ca_{1.6}SiO_{3.6}:2.58H_2O = 1.6Ca^{+2} - 3.2H^{+} + 1H_4(SiO_4) + 2.18H_2O$	28.00		
CSH1.2	71.95	$Ca_{1,2}SiO_{3,2}:2.06H_2O = 1.2Ca^{+2} - 2.4H^{+} + 1H_4(SiO_4) + 1.26H_2O$	19.30		
СЗАН6	149.52	$Ca_3Al_2(OH)_{12} = 3 Ca^{+2} + 2 Al^{+3} - 12 H^+ + 12 H_2O$	80.32		
C3FH6	154.50	$Ca_3Fe_2(OH)_{12} = 3 Ca^{+2} + 2 Fe^{+3} - 12 H^+ + 12 H_2O$	72.39		
C4AH13	269.20	$Ca_4Al_2(OH)_{14}:6H_2O = 4 Ca^{+2} + 2 Al^{+3} - 14 H^+ + 20 H_2O$	103.65		
C4FH13	274.4	$Ca_4Fe_2(OH)_{14}:6H_2O = 4 Ca^{+2} + 2 Fe^{+3} - 14 H^+ + 20 H_2O$	95.16		
Ettringite-Fe	711.80	$Ca_{6}Fe_{2}(SO_{4})_{3}(OH)_{12}:26H_{2}O = 6 Ca^{+2} + 2 Fe^{+3} - 12 H^{+} + 3 SO_{4}^{-2} + 38 H_{2}O$	54.55		
Gypsum	74.69	$CaSO_4:2H_2O = 1 Ca^{+2} + 1 SO_4^{-2} + 2 H_2O$	-4.61		
Hemicarboaluminate	569.02	$Ca_6O_6:Al_4O_6:CaCO_3:Ca(OH)_2:21H_2O = 8 Ca^{+2} + 4 Al^{+3} - 26 H^{+} + 1 CO_3^{-2} + 35 H_2O$	173.20		
Hydrotalcite-CO ₃	231.44	$Mg_4Al_2(OH)12CO_3:2H_2O = 4 Mg^{+2} + 2 Al^{+3} - 12 H^+ + 1 CO_3^{-2} + 14 H_2O$	50.86		
Fe(OH) ₂ (cr)	24.48	$Fe(OH)_2 = 1 Fe^{+2} - 2 H^+ + 2 H_2O$	12.76		
Monocarboaluminate	261.96	$(CaO)_{3}Al_{2}O_{3}:CaCO_{3}:10.68H_{2}O = 4 Ca^{+2} + 2 Al^{+3} - 12 H^{+} + 1 CO_{3}^{-2} + 16.68H_{2}O$	70.30		
Monosulfate-Fe	316.06	$Ca_4Fe_2(SO_4)(OH)_{12}:6H_2O = 4 Ca^{+2} + 2 Fe^{+3} - 12 H^+ + 1 SO_4^{-2} + 18 H_2O$	66.05		
Monosulfoaluminate	311.26	$Ca_4Al_2(SO_4)(OH)_{12}:6H_2O = 4 Ca^{+2} + 2 Al^{+3} - 12 H^{+} + 1 SO_4^{-2} + 18 H_2O$	73.07		
Portlandite	33.06	$Ca(OH)_2 = 1 Ca^{+2} - 2 H^+ + 2 H_2O$	22.81		
Pyrrhotite	18.20	$Fe_{0.87}S = 0.87Fe^{+2} - 0.74H^{+} + 1 HS^{-} - 0.13H_2O + 0.065O_2$	-11.18		
Saponite-FeCa	139.96	$Ca_{0.17}Mg_2FeAl_{0.34}Si_{3.66}O_{10}(OH)_2 = 0.17Ca^{+2} + 2 Mg^{+2} + 1 Fe^{+2} + 0.34Al^{+3} - 7.36H^{+} + 3.66H_4(SiO_4) - 2.64H_2O$	26.55		
Syngenite	151.63	$K_2Ca(SO_4)_2:6H_2O = 1 Ca^{+2} + 2 K^+ + 2 SO4^{-2} + 6 H_2O$	-7.45		

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928 **7.2** Description and results of preliminary cases

929 7.2.1 P1: Tracer diffusion (without considering geochemistry)

930 Case P1 studied the diffusion of a non-reactive and uncharged tracer from the concrete domain 931 towards the clay host rock in the setup presented in Fig. 1 and Table 1 and the same spatial 932 discretization than the FRC. The initial tracer concentration in the concrete and claystone pore 933 solutions is 1 and 0 mM, respectively (Table 4). A closed boundary condition is imposed on the left 934 and a tracer concentration of 0 on the right. In addition, two alternative reactive transport codes 935 were included in the comparison, namely PHAST (Parkhurst et al., 2010) and NPS (Damiani et al., 936 2019).

937 Fig. 13 shows the concentration profiles obtained with several codes at different times. Diffusion of 938 the tracer towards the claystone has the effect of decreasing its concentration in concrete as a 939 function of time. Good comparison between six codes is found for the tracer diffusion. After 100 940 years, slightly lower concentrations can be found in the concrete domain with MIN3P.



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944 7.2.2 P2: Aqueous species and cation exchange

945 Case P2 is equivalent to the FRC but without any primary or secondary minerals, i.e. only the 946 aqueous solutions and cation exchange reactions are considered. The goal was to benchmark the 947 implementation of cation exchange reactions before implementing mineral reactions. Concrete/clay 948 interaction is thus limited to porewater diffusion and cation exchange in thermodynamic 949 equilibrium. This case was simulated with iCP, MIN3P, OGS-GEM and CORE^{2D}. Results after 950 1,000 years of interaction centred on the first meter of the studied system are presented from Fig. 14 to Fig. 16. Concrete exchanger concentrations of Ca²⁺, K⁺ and Na⁺ at 1,000 years (Fig. 14). K⁺ 951 952 release (initial concentration of 0.55 mol/L) to concrete porewater driven by diffusion towards the 953 claystone is the main process. This decrease in K⁺ fraction in the exchanger is compensated by an increase in Na⁺ (initial concentration of 0.17 mol/L), diffusing from the claystone, and in Ca²⁺. Ca²⁺ 954 concentration remains unchanged at x = 0 (initial concentration of 0.45 mol/L). Na⁺ and Ca²⁺ 955 956 concentrations changes in the exchanger maintain a similar Ca/Na ratio in the concrete exchanger. 957 Overall, good agreement between codes is found, with slight differences in the results of MIN3P. In 958 turn, Fig. 15 shows the composition of the exchanger within the first meter of claystone. Potassium 959 diffusion from concrete results in an important concentration increase in the clay exchanger. As a consequence, all other cation concentrations are reduced. 960

Total dissolved calcium and potassium distribution profiles at 1,000 years show the in-diffusion into concrete of the former from the clay and the out-diffusion of the latter (Fig. 16). After 1,000 years, only small variations are observed in the pH profile (Fig. 16), with values between 7.1 and 7.2 for the two materials (initial pH is 10.68 and 7.06 for concrete and clay, respectively). As the clayey domain has a much larger volume (\sim 130 times) than the concrete one, pH tends to the clay initial values in the system. Differences in porewater between codes can be directly linked with the differences observed in the exchanger compositions.



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969 *Fig. 14.* Cation concentration (M) profiles in concrete exchanger at 1,000 years obtained with iCP, MIN3P,
970 OGS-GEM and CORE^{2D}.

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Fig. 16. Total aqueous concentrations (M) and pH profiles at 1,000 years obtained with iCP, MIN3P, OGS GEM and CORE^{2D}.

979 **7.2.3 P3: Minerals in equilibrium**

The setup of case P3 is equivalent to the FRC but without accounting for the kinetically-controlled 980 981 minerals. The only exception is quartz, which is included in case P3 as secondary phase in 982 thermodynamic equilibrium in both materials, although in the FRC it was considered as a kinetically-controlled mineral. Quartz is expected to form in the model when the C-S-H gel 983 dissolves. This case was simulated with iCP, MIN3P and CORE^{2D}. It is to note that in MIN3P, 984 mineral reactions can only be simulated using kinetic rates. Thus, fast kinetic rates (1.10⁻⁹ 985 986 mol/L_{medium}/s) were considered for all minerals to mimic thermodynamic equilibrium conditions. The results after 100,000 years are presented in terms of pH and porosity (Fig. 17) and mineral 987 profiles (Fig. 18 and Fig. 19). The results of CORE^{2D} are the same as those included in the FRC 988 989 comparison.

990 Overall, a very good agreement is observed between the results obtained with the different codes, 991 with very small differences in pH and porosity profiles. However, the sensitivity of the system to 992 iron is noticeable in the magnetite and siderite profiles.





996 0 0.1 x (m) 0.2 0.3 0 0.1 x (m) 0.2 0.3 0 0.1 x (m) 0.2 0.3 0 0.1 x (m) 0.2 997 Fig. 18. Mineral phase profiles (in mol/ L medium) in the concrete domain at 100,000 years, obtained with 998 iCP, MIN3P and CORE^{2D}.



 0 $^{0.3}$ $^{0.6}$ x (m) $^{0.9}$ $^{1.2}$ $^{1.5}$ 0 $^{0.3}$ $^{0.6}$ x (m) $^{0.9}$ $^{1.2}$ $^{1.5}$ 0 $^{0.3}$ $^{0.6}$ x (m) $^{0.9}$ $^{1.2}$ $^{1.5}$ 0 $^{0.3}$ $^{0.6}$ x (m) $^{0.9}$ $^{1.2}$ $^{1.5}$ 1001Fig. 19. Mineral phase profiles (in mol/ L medium) in concrete and clay domains at 100,000 years, obtained1002with iCP, MIN3P and CORE^{2D}.

Highlights

• Hydration and long-term performance of a newly developed low-pH concrete based on blending cement, silica fume and blast furnace slag are assessed by means of numerical modelling.

• The hydrated concrete composition is used to study the long-term interaction (100 000 years) between this cementitious system and Callovo Oxfordian claystone.

• Several reactive transport codes are benchmarked including complex geochemical processes (precipitation/dissolution reactions thermodynamically- and kinetically-controlled, cation exchange processes and redox reactions) and couplings.

• The results show a good level of agreement and provide confidence on the use of reactive transport modelling for evaluation of the longterm safety of cementitious materials in repositories for disposal of radioactive waste.

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Declaration of interests

 \boxtimes The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

The authors declare the following financial interests/personal relationships which may be considered as potential competing interests:

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